

ALASKA RAILROAD CORPORATION

ENGINEERING SERVICES P.O. BOX 107500, ANCHORAGE, ALASKA 99510-7500

BR. 25.4 REPLACEMENT

LATITUDE: 60.4303°, **LONGITUDE:** -149.3714°

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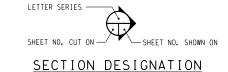
REFERENCES

I STANDARD 28 FOOT SPAN CONCRETE BALLAST DECK (CBD)

READY TO ADVERTISE

ALASKA RAILROAD CORPORATION

P.O. BOX 107500, ANCHORAGE, ALASKA 99510-7500



BR. 25.4 REPLACEMENT

COVER SHEET

SCALE: AS NOTED NAS/DTP

I OF 12

DRAWN BY: CHECKED BY: BWB/DAO DATE: 04/08/22 APPROVED BY:

GENERAL NOTES

- I. All work requirements on these drawings and not otherwise detailed shall be accomplished as specified in the current edition of the American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual for Railway Engineering.
- Field verify all dimensions and elevations prior to start of construction.
- Stationing for these plans can be converted to Br. 25.4 Rail Raise Track Plans by adding 1171+75.43.
- 4. Information used to prepare this drawing:

Geotechnical investigation and recommendations prepared by Northern Geotechnical Engineering, Inc. d.b.a. Terra Firma Testing, Dated 04/07/20.

Hydraulic and hydrologic investigation and recommendations prepared by Michael Baker International, Dated 06/10/20.

DESIGN NOTES

- The proposed structure has been designed in accordance with the AREMA Manual for Railway Engineering, Chapter 8: Concrete Structures and Foundations, Chapter 9: Seismic Design for Railway Structures and Chapter 15: Steel Structures.
- 2. This structure was designed for Cooper E80 Live Load plus Impact.

FIELD WELDING

- I. Welding shall be accomplished with the SMAW or FCAW Process.
- Welding shall be in compliance with the requirements specified in AWS DI.5, except %" fillet welds may be made with a single pass.
- 3. Welding electrodes shall be E7018 for SMAW or E71T-7 for FCAW.
- 4. Welders shall possess valid qualifications, which shall be furnished to the Railroad for approval prior to commencing welding.

CAST-IN-PLACE CONCRETE NOTES

CONCRETE

- I. All concrete materials, placement and workmanship shall be in accordance with Chapter 8: Concrete Structures and Foundations of the AREMA Manual for Railway Engineering. This section of notes applies to all concrete work except for drilled shafts.
- 2. Formwork tolerances shall be in accordance with ACI 347 specifications.
- 3. Minimum compressive strength at 28 days shall be 4000 psi.
- Concrete shall be proportioned such that the water cement ratio (by weight) does not exceed 0.45.
- Cement shall be Type I or Type II Portland Cement, blended hydraulic cement, fly ash, ground granulated blast-furnace slag, or silica fume in accordance with AKDOT 501 specifications.
- 6. Aggregates shall be graded in accordance with AKDOT Standard Specifications for Highway Construction 2020 Edition.
- 7. Air content shall be between 5.5% and 6.5% (by volume).
- 8. Admixtures shall not be used without approval by the Railroad.
- 9. Curing shall be accomplished by wet curing or application of a Type 2 membrane.
- 10. Concrete work shall conform to all requirements of ACI 306.1, Standard Specification for Cold Weather Concreting. Contractor shall submit detailed procedures for the production, transportation, placement, protection, curing, and temperature monitoring of concrete during cold weather to the Railroad for approval.

REINFORCING STEE

- Reinforcing steel shall be deformed, new billet bars per ASTM A615 specifications and meet Grade 60 requirements. Bars shall be non-coated.
- Fabrication of reinforcing steel shall be per Chapter 7 of the CRSI Manual of Standard Practice. Dimensions of bending details are out to out of bar.
- 3. Reinforcing steel shall be blocked and tied to proper location and securely wired against displacement. Tile wires shall be installed at every other bar intersection so that at least 50% of the intersections are tied. Tack welding of reinforcing is prohibited. Minimum concrete cover on reinforcing not otherwise noted shall meet the AREMA Manual for Railway Engineering requirements.

PILE DRIVING NOTES

<u>DESIGN</u>

- I. All piles shall be driven to 100 ton capacity.
- Minimum pile penetration depth = 35' below groundline. Estimated pile driving depth = 54' below groundline.
- Estimated capacity of driven piles shall be calculated using the Modified ENR formula, with Factor of Safety of 5. Pile driving records and estimated capacities shall be submitted to the engineer. Alternate methods such as Gates or PDA testing may also be considered at the Railroad's discretion.
- 3. Vibratory hammers are only permitted for 20' max of pile advancement below the mudline.
- 4. Mark every pile with a dimension indicating the pile depth from cutoff to point of pile. The dimension shall be rounded to the nearest foot. The mark shall be welded on the outside face, low mile post side on the pile flange, approximately 1'-0" below the bottom of the cap, and in numbers of approximately 3" in height. If a pile is not exposed, no mark is required.

STEEL

- Piles ASTM A252, Grade 3, split seam welded. Upon approval, split seam welded pipe conforming to; ASTM 53 Grade B, API Specification 2B, or API Specification 5L X52 PSL2 may be used.
- 2. Pile splices ASTM A572 Grade 50.
- 3. Backing rings provided by manufacturer or an approved alternate equal to a minimum thickness of $\frac{1}{4}$ " and a width of 4".

GALVANIZED PIPE PILE

- Where specified, galvanized coating for pipe piles shall conform to ASTM A123. Pickle per SSPC No. 8 and Hot-Dipped Galv. per current ASTM A123. Coating weight 2.3 oz. per sq. ft. (Grade 100).
- 2. Provide 3" Mask on each end of Pile

SPLICES.

I. Splices shall be made a sufficient distance above the ground or water (not less than one foot) so that the splice can be observed during driving. The number of splices shall be kept to a minimum. Splicing cut-offs or short pieces to make a main bearing pile is not permitted. The pile shall be driven so that the upper splice is at least 10 feet below the ground surface.

TOLERANCE

I. Variations greater than ¼" per foot from vertical or batter line shall not be allowed. The deviation of the top of the piles in a bent shall not exceed one inch from the plan location. Piles not meeting tolerance requirements or out of line as to impair usefulness, or piles that are damaged in driving as to impair structural capacity, shall be pulled and redriven or an additional pile shall be driven to provide added support.

MISCELLANEOUS STEEL SPECIFICATIONS:

 $\hbox{Design and Workmanship - Per current AREMA Manual for Railway Engineering.}$

Miscellaneous Steel - Per current ASTM A36 Specifications, unless otherwise noted.

Steel Coating - Unless otherwise noted, pickle per SSPC No. 8 and Hot-Dipped Galv. per current ASIM A123 (Grade 100). Coating weight 2.3 oz. per sq. ft. Bolts and nuts to be zinc plated.

Welding - Arc Process per current AREMA Manual for Railway Engineering and AWS DI.I Structural Welding Code.

RIPRAP SPECIFICATIONS:

- I. Riprap shall be placed to thickness, height and length shown on plans. Individual stones are to be keyed into each other to reduce void space in the finished product.
- Riprap subgrade shall be prepared to achieve the lines and grades shown on the plans prior to riprap placement. Any fill required in the subgrade shall be compacted to a density consistent with surrounding undistrurbed material.

WOVEN FILTER FABRIC SPECIFICATIONS:

- I. Woven filtration fabric shall be installed between all riprap and underlying soil. Fabric material shall meet AASHTO M288 specifications for stabilization.
- Site preparation shall include removing vegetation, large stones, limbs, brush, roots and other debris.
- Sheets placed in the improved bank and channel shall be orientated with the longer dimension parallel with the embankment.
- 4. Upper sheets shall overlap the lower sheets with a minimum 18 inch overlap between individual fabric sheets.
- 5. Include folds in fabric to minimize tension and stretching during settlement.
- 6. Secure fabric with pins and washers at 2 to 5 foot spacing along the overlaps.
- 7. Do not drop stones from heights greater than 2 feet when placing riprap on filter

SUMMARY OF ESTIMATED QUANTITIES						
DESCRIPTION	ESTIMATING UNIT	QUANTITY				
PRECAST CONCRETE THREE PILE, SINGLE ROW BENT CAP (PER REF. 1, DWG. NO. 8-9)	EA.	3				
PRECAST CONCRETE TWO ROW ABUTMENT CAP (PER REF. I, DWG. NO. 11-12)	EA.	2				
27'- ½" CONCRETE BALLAST DECK SPAN WITH DIAPHRAGMS SHOP INSTALLED (PER REF. I, DWG. NOS. 3-6)	EA.	8				
DIAPHRAGM CHANNEL ASSEMBLY FOR TANGENT TRACK ALIGNMENT (PER REF. 1, DWG. NO. 14)	EA.	12				
U% "×10"× 1'-4½" LAMINATED BEARING PAD (PER REF. 1, DWG. NO. 4)	EA.	32				
EXPANSION BEAM STOP ASSEMBLY (PER REF. I, DWG. NO. 3)	EA.	8				
FIXED BEAM STOP ASSEMBLY (PER REF. I, DWG. NO. 3)	EA.	8				
I½" DIA. x I'-9" ANCHOR ROD, HOT DIP GALVANIZED, ASTM F1554 GR. 36	EA.	16				
I" DIA. × 2½" A325 BOLT, HOT DIP GALVANIZED	EA.	32				
STEEL TEE T-20 (PER DETAILS, DWG, NO, 10)	EA.	12				
INTERIOR DECK PLATE DP-1 (PER DETAILS, DWG. NO. 10)	EA.	6				
EXTERIOR DECK PLATE DP-3 (PER DETAILS, DWG. NO. 10)	EA.	2				
15" TIE PLATE (FOR 51/2" RAIL BASE)	EA.	208 **				
¼" × 7¾" × 15" TIE PAD, CORDED RUBBER, DUROMETER 70	EA.	208 **				
RIGHT HAND "E" CLIP	EA.	406 **				
15% " DIA. × 6½" RECTANGULAR HEAD TIMBER COACH SCREW (HIGH TENSILE, 120 KSI MIN.)	EA.	812 **				
WALKWAY AND HANDRAIL MATERIAL SCHEDULE (PER DETAILS, DWG. NO. 12)	LOT	1				
Ι¼"x6"x 12'-4" UHMW PLASTIC BULKHEAD CAP BEARING PAD (PER REF. I, DWG. NO. II)	EA.	2				
⅓" DIA. x 6" HDG ANCHORS (PER REF. I, DWG. NO. II)	EA.	16				
GALVANIZED STEEL PIPE PILE 12" DIA. x ½" THICKNESS, 40'-0" LENGTH, ONE END BEVELED, (ASTM A252, GRADE 3) (SHOP GALVANIZE PER NOTES DWG. NO. 2)	EA.	12				
GALVANIZED STEEL PIPE PILE 12" DIA. x ½" THICKNESS, 60'-0" LENGTH, NO END BEVELS, WITH CUTTING SHOE 0-14001, OR APPROVED ALTERNATE, SHOP ATTACHED (ASTM A282, GRADE 3)	EA.	12				
GALVANIZED STEEL PIPE PILE 16" DIA. x ½" THICKNESS, 40'-0" LENGTH, ONE END BEVELED, (ASTM A252, GRADE 3) (SHOP GALVANIZE PER NOTES DWG. NO. 2)	EA.	9				
GALVANIZED STEL PIPE PILE 16" DIA. x ½" THICKNESS, 60'-0" LENGTH, NO END BEVELS, WITH CUTTING SHOE 0-14001, OR APPROVED ALTERNATE, SHOP ATTACHED (ASTM AZ82, GRADE 3)	EA.	9				
4,000 PSI CONCRETE FOR PIPE PILES (PER DETAILS, DWG. NO .7)	CU. YD.	22.3				
FLOWABLE FILL FOR PIPE PILE FILL (PER DETAILS, DWG. NO .7)	LOT	1				
REINFORCING STEEL FOR PIPE PILES (PER SCHEDULES, DWG. NO. 7)	LOT	1				
BACKING RING, APF S-40000 D=11.0" (PER NOTES, DWG. NO. 2 AND DETAILS, DWG NO. 7)	EA.	12				
BACKING RING, APF S-40000 D=15.0" (PER NOTES, DWG. NO. 2 AND DETAILS, DWG NO. 7)	EA.	9				
CUTTING SHOE, APF 0-14001 FOR 12" DIA. PIPE (PER DETAILS, DWG NO. 7)	EA.	12				
CUTTING SHOE, APF 0-14001 FOR 16" DIA. PIPE (PER DETAILS, DWG NO. 7)	EA.	9				
BULKHEAD MATERIAL (PER SCHEDULE, DWG NO. 9)	LOT	ı				
GEOTEXTILE FABRIC (PER AASHTO M288 SPECIFICATIONS)	LOT	1				
RIPRAP, CLASS III	TON	280				
NON-CEMENTITIOUS GROUT, SIKADUR 32 OR APPROVED EQUAL	LOT	1				
COLD GALVANIZING COMPOUND SPRAY	LOT	<u> </u>				
EST. WT. OF STEEL SHEFT PILING = 152,700 LB.						

EST. WT. OF STEEL PIPE PILING = 152,700 LB. EST. WT. OF STEEL SHEET PILING = 107,530 LB. EST. WT. OF MISC. STEEL = 4,530 LB.

OUANTITIES PROVIDED FOR ESTIMATING AND PLANNING PURPOSES ONLY. CONTRACTOR IS RESPONSIBLE TO FURNISH ALL PROJECT MATERIAL TO MEET PLAN REQUIREMENTS.

- * BALLAST AND TRACK MATERIALS PROVIDED BY ARRC
- * CURRENT SINGLE PILE ROW BENT CAPS IN INVENTORY (9) HAVE ONLY TWO (2) DYWIDAG BLOCKOUTS FOR ANCHORS. TWO (2) ADDITIONAL HOLES WILL NEED TO BE FIELD CORED.
- "**" INDICATES ADDITIONAL 5% (OR MINIMUM OF I EXTRA) INCLUDED IN QUANTITY.



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ENGINEERING SERVICES

P.O. BOX 107500. ANCHORAGE ALASKA 99510-7500

BR. 25.4 REPLACEMENT

GENERAL NOTES AND BILL OF MATERIAL

DESIGNED BY: NAS /MIR DWG NO

DESIGNED BY: NAS/MLB SCA
DRAWN BY: NAS/DTP
CHECKED BY: BWB/DAO DAT

REV. DATE BY

REVISION

SCALE: AS NOTED

DATE: 04/08/22

2 OF 12



CONTROL POINT LIST

Point	Northing	Easting	Elevation	Description
126	2349093.98	1754096.83	485.55	PK IN TIE
206	2349593.05	1754017.98	485.50	60D IN TIE
2037	2349710.76	1754008.93	484.58	5/8" RBAR W/ YPC
2039	2349853.10	1753947.78	484.45	5/8" RBAR W/ YPC
416	2350395.26	1753891.33	479.86	PK IN TIE
101	2350576.08	1753821.89	480.22	PK IN ASPH
572	2351083.35	1753783.13	475.69	PK IN TIE
1956	2351471.24	1753757.48	475.55	5/8" RBAR W/ YPC
869	2351848.53	1753820.61	474.80	5/8" RBAR W/ YPC
992	2352322.34	1753858.34	476.05	set pk in tie
1092	2352915.87	1753804.90	475.68	PK IN TIE
1170	2353415.22	1753762.59	475.87	PK IN TIE
2043	2353909.77	1753854.42	474.33	5/8" RBAR W/ YPC
1288	2354202.35	1753876.88	475.48	PK IN TIE
1405	2354775.71	1754237.58	475.38	PK IN TIE
1486	2355205.57	1754463.53	475.87	PK IN TIE
1559	2355692.60	1754392.09	476.46	PK IN TIE
1644	2356259.93	1754222.02	477.35	PK IN TIE
1678	2356496.51	1754253.72	477.55	PK IN TIE
1679	2357143.05	1754375.59	474.94	PK IN TIE
1749	2357545.35	1754451.36	474.02	60D IN TIE
2045	2357625.27	1754479.67	473.31	5/8" RBAR W/ YPC
1818	2358080.86	1754549.98	476.64	PK IN TIE
100	2350580.89	1753825.35	479.98	BASE OPUS

NOTES

1) HORIZONTAL AND VERTICAL DATUM IS ALASKA STATE PLANE
COORDINATE SYSTEM ZONE 4, NAD83(2011), GEOID12B ORTHOMETRIC
HEIGHTS BASED ON OPUS PROCESSSING OF CONTROL POINT 100.
2) CONTROL POINT 100 IS NOT SHOWN IN PLAN ABOVE.

LEGEND

CONTROL POINT, SEE POINT LIST

APPROXIMATE LOT LINES PER KPB GIS

REV. DATE BY

REVISION

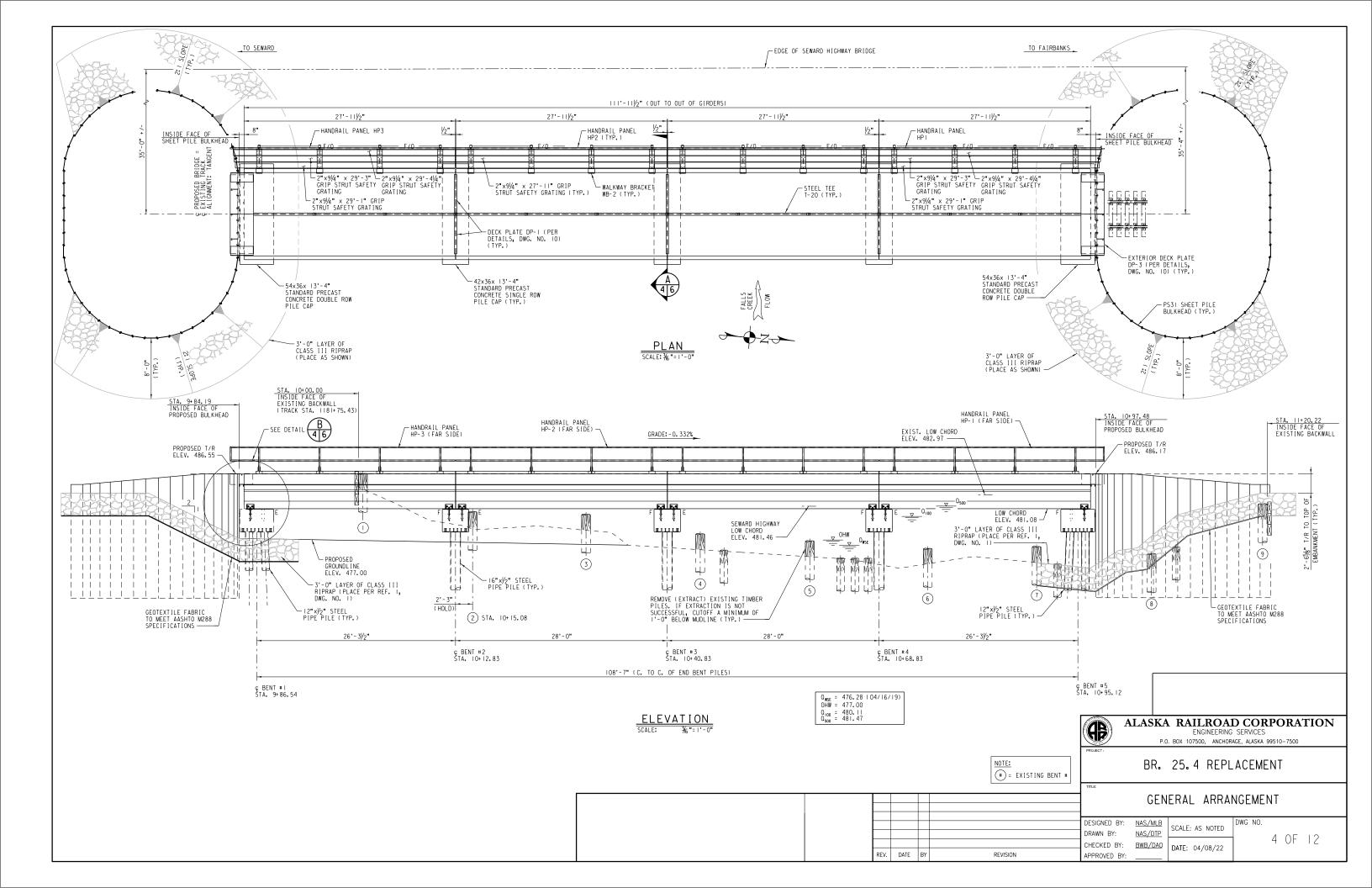


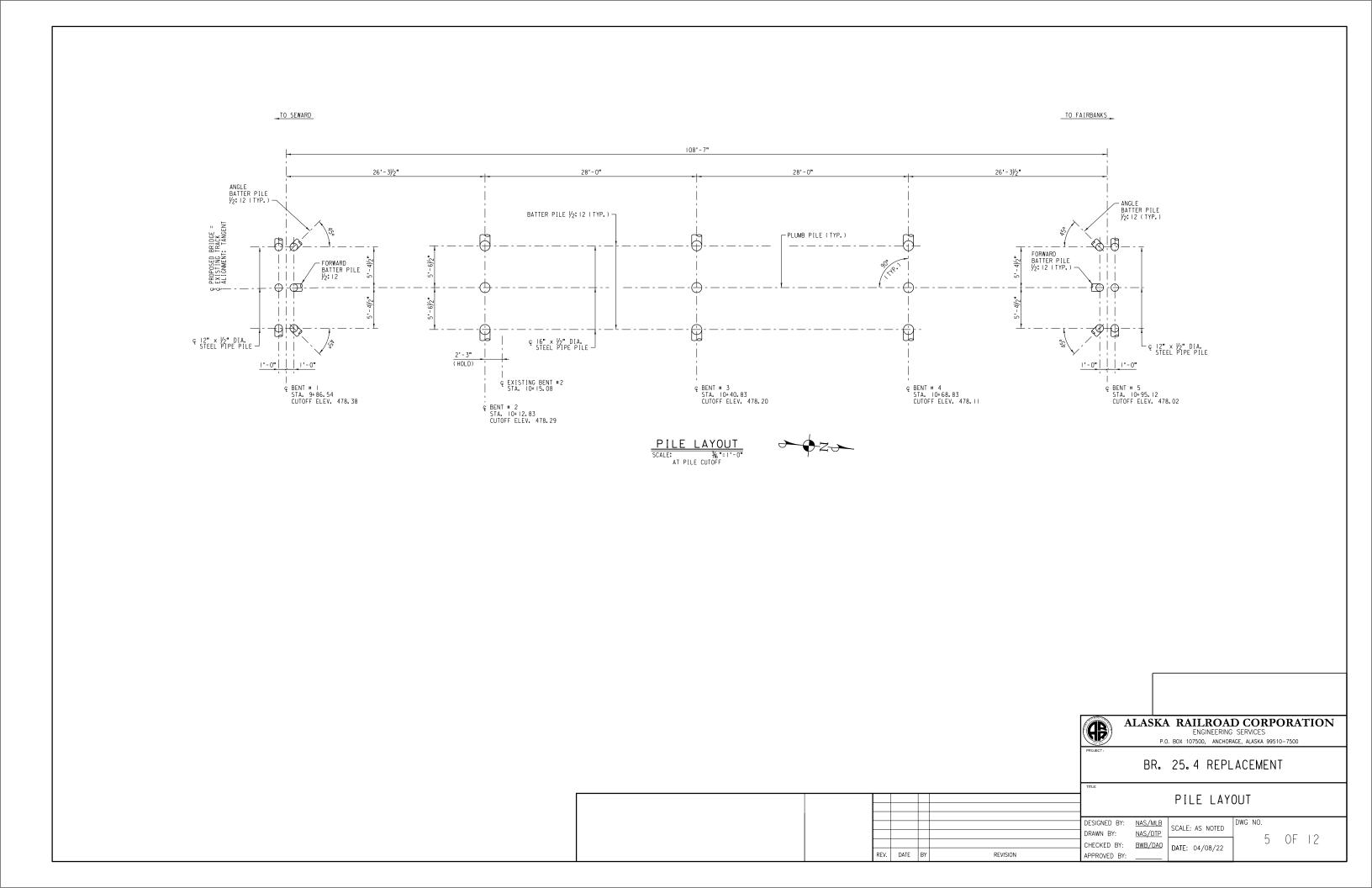
BR. 25.4 REPLACEMENT

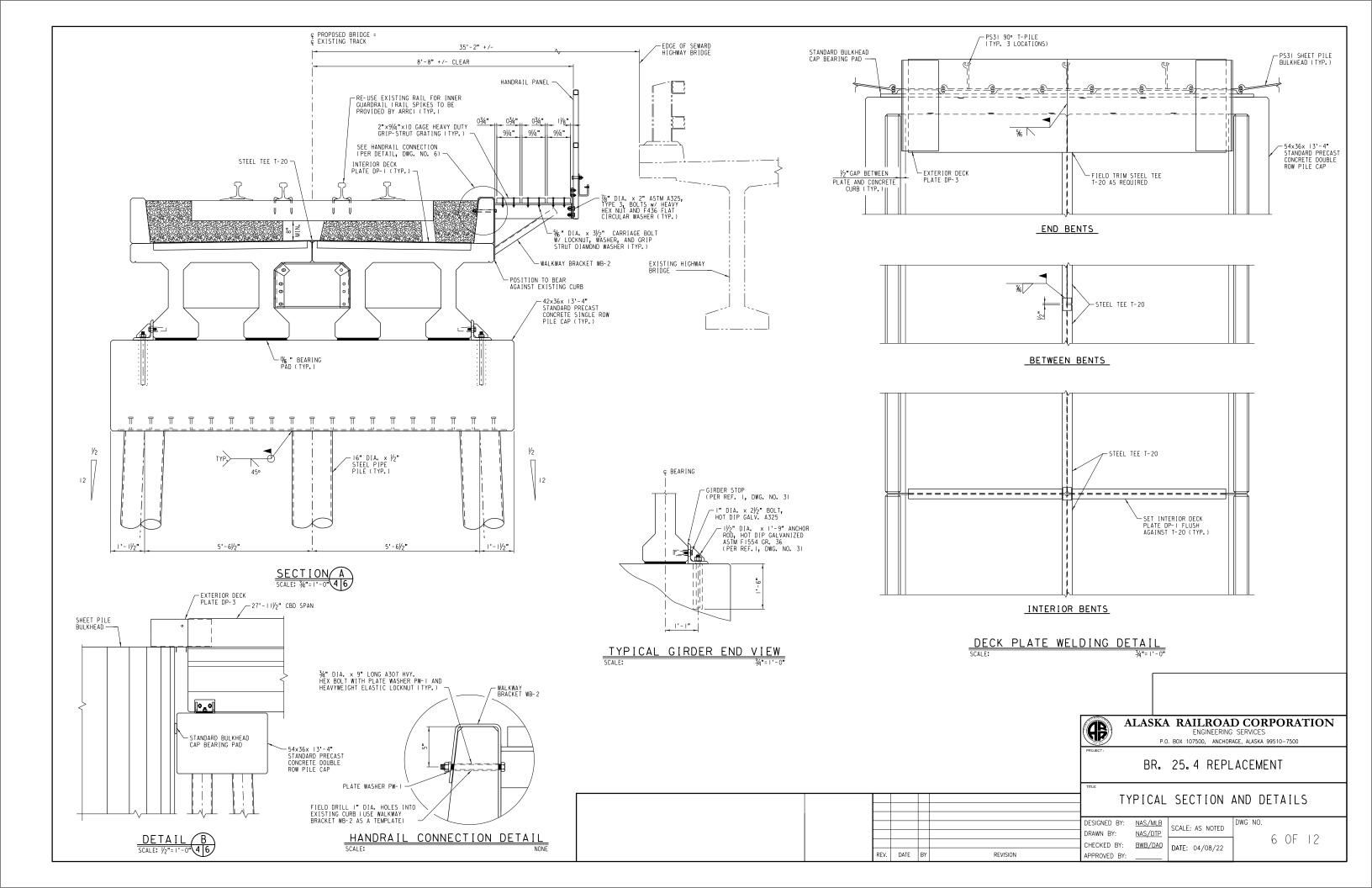
SURVEY CONTROL

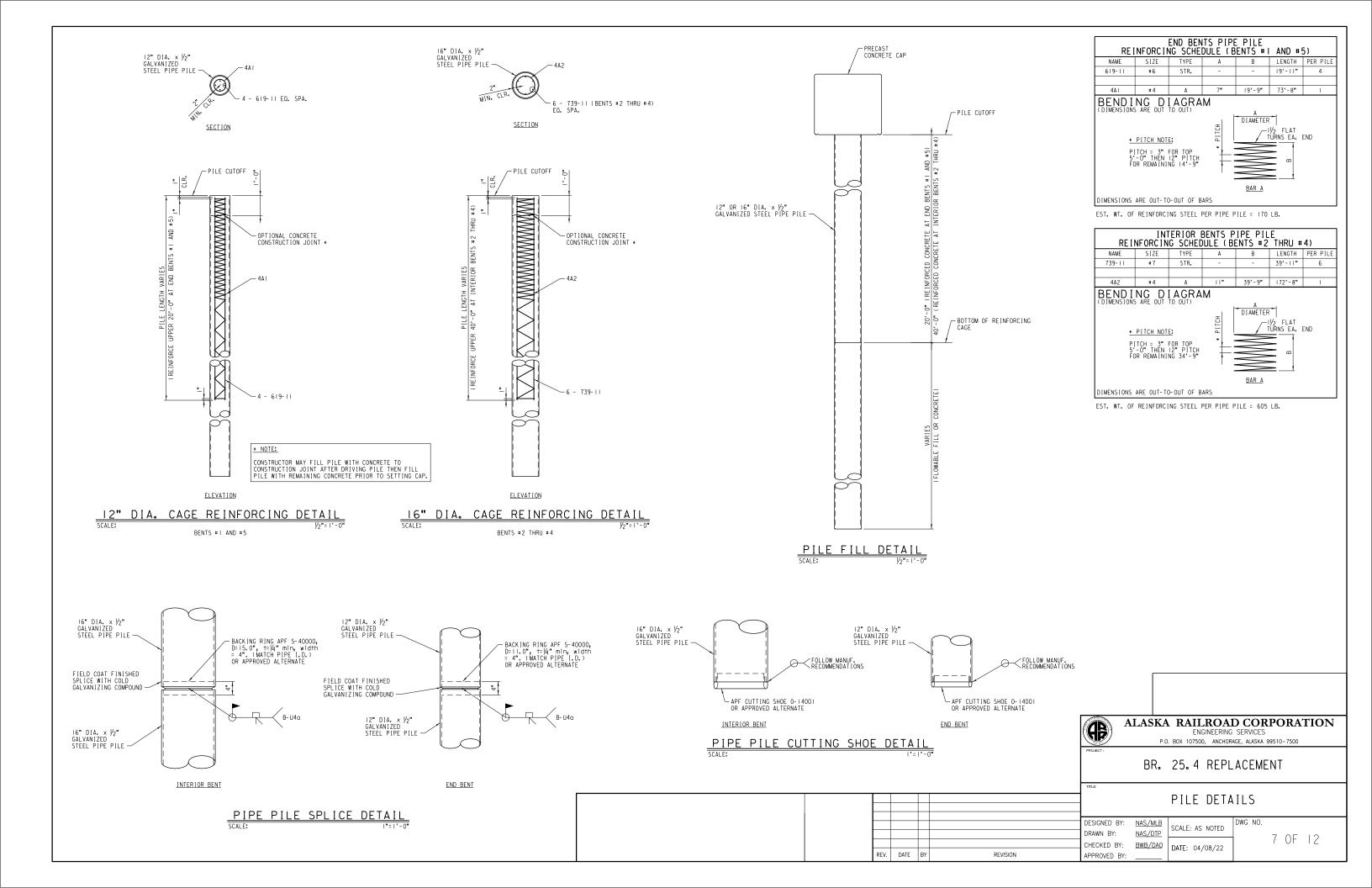
DESIGNED BY:	NAS/MLB	SCALE: AS NOTED	DWG	NC
DRAWN BY:	NAS/DTP	SCALL. AS NOTED		

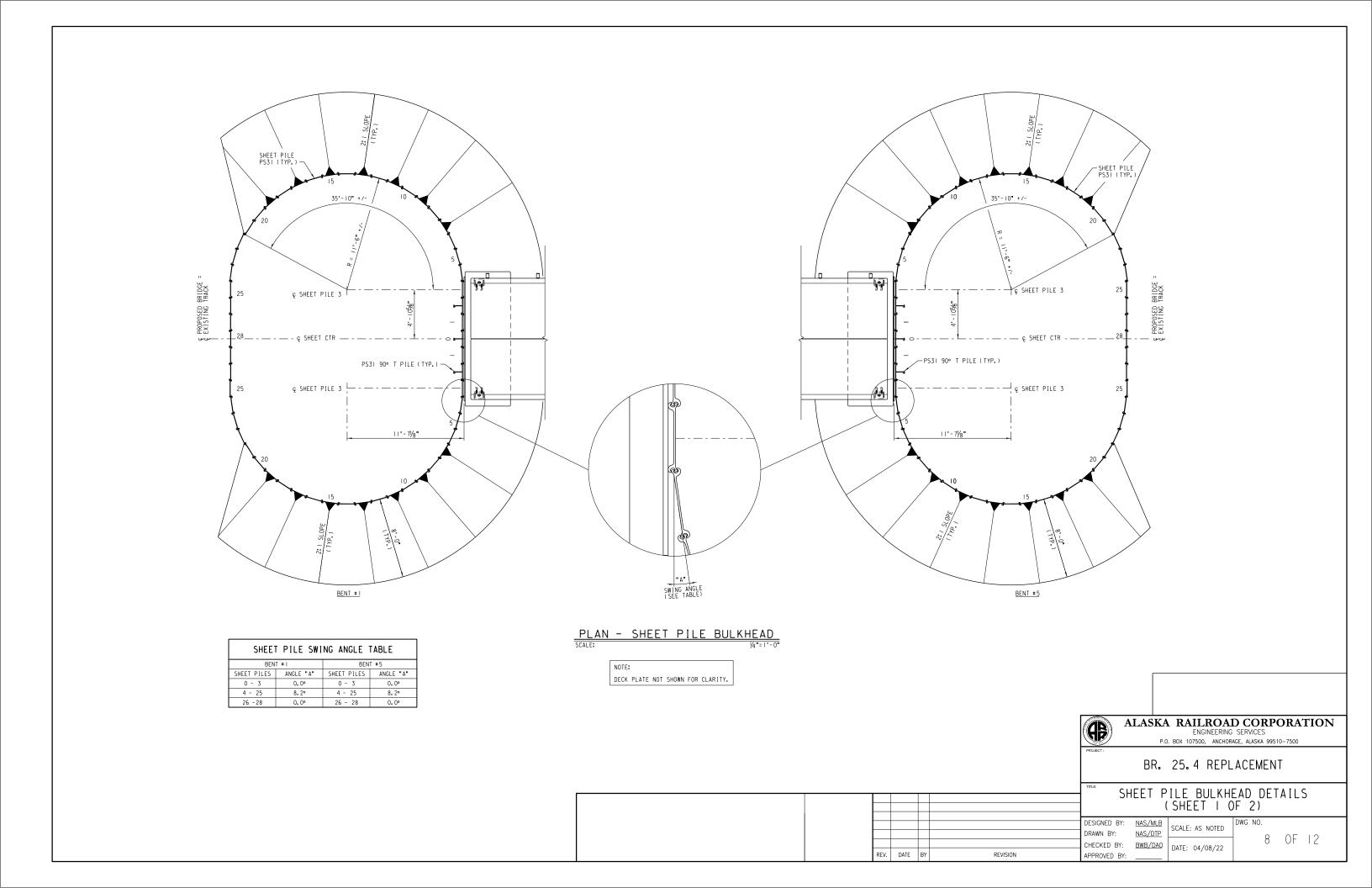
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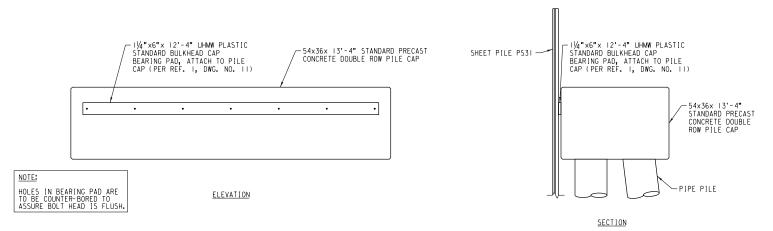






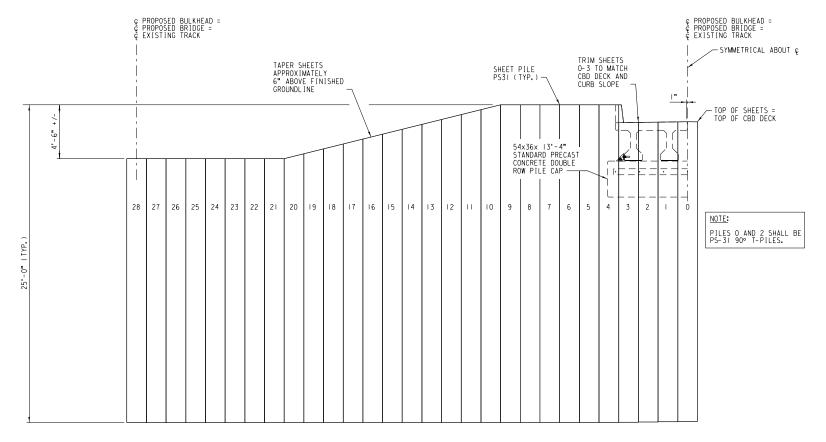






BULKHEAD BEARING ASSEMBLY DETAIL

SCALE: 1/2"=1'-0



BENT # I AND BENT #5

BULKHEAD ELEVATION
SCALE: 1/4'=1'-0"

NOTE:

ALL SHEET TO BE DRIVEN TO HIGHER ELEVATION AND CUT DOWN TO FINAL ELEVATION, REMOVING LIFTING HOLES IN SHEET PILE.

BULKHEAD MATERIAL SCHEDULE

REO'D. UNIT DESCRIPTION

106 EA. PS31 SHEET PILE, 25'-0" LENGTH (ASTM A572, GRADE 50)

6 EA. PS31 90° T PILE, 25'-0" LENGTH (ASTM A572, GRADE 50)

EST. WT. OF SHEET PILING = 107,530 LB.

NOTES:

- I. FILL INSIDE BULKHEAD SHALL BE STRUCTURAL FILL FURNISHED, SHAPED, AND COMPACTED IN ACCORDANCE WITH AKDOT AND PUBLIC FACILITIES STANDARD SPECIFICATION FOR HIGHWAY CONSTRUCTION 2020 EDITION, SECTION 205, EXCAVATION AND FILL FOR MAJOR STRUCTURES.
- 2. SUBBALLAST ABOVE STRUCTURAL FILL SHALL BE A MINIMUM I'-O" GRADING C-I FURNISHED, PLACED, SHAPED, AND COMPACTED TO AKDOT AND PUBLIC FACILITIES STANDARD SPECIFICATION FOR HIGHWAY CONSTRUCTION 2020 EDITION, SECTION 301, AGGREGATE BASE AND SURFACE COURSE.
- . AFTER INSTALLATION OF BULKHEAD SHEET PILES CONTRACTOR SHALL FIELD DRILL I" DIAMETER DRAIN HOLES IN THE SHEET PILES I'-O" ABOVE THE GROUND SURFACE ON THE OUTSIDE OF THE SHEET PILES. DRAIN HOLES SHALL BE A MAXIMUM OF 5'-O" SPACING LONGITUDINALLY AROUND THE CONSTRUCTED BULKHEAD.



APPROVED BY:

REV. DATE BY

REVISION

ALASKA RAILROAD CORPORATION

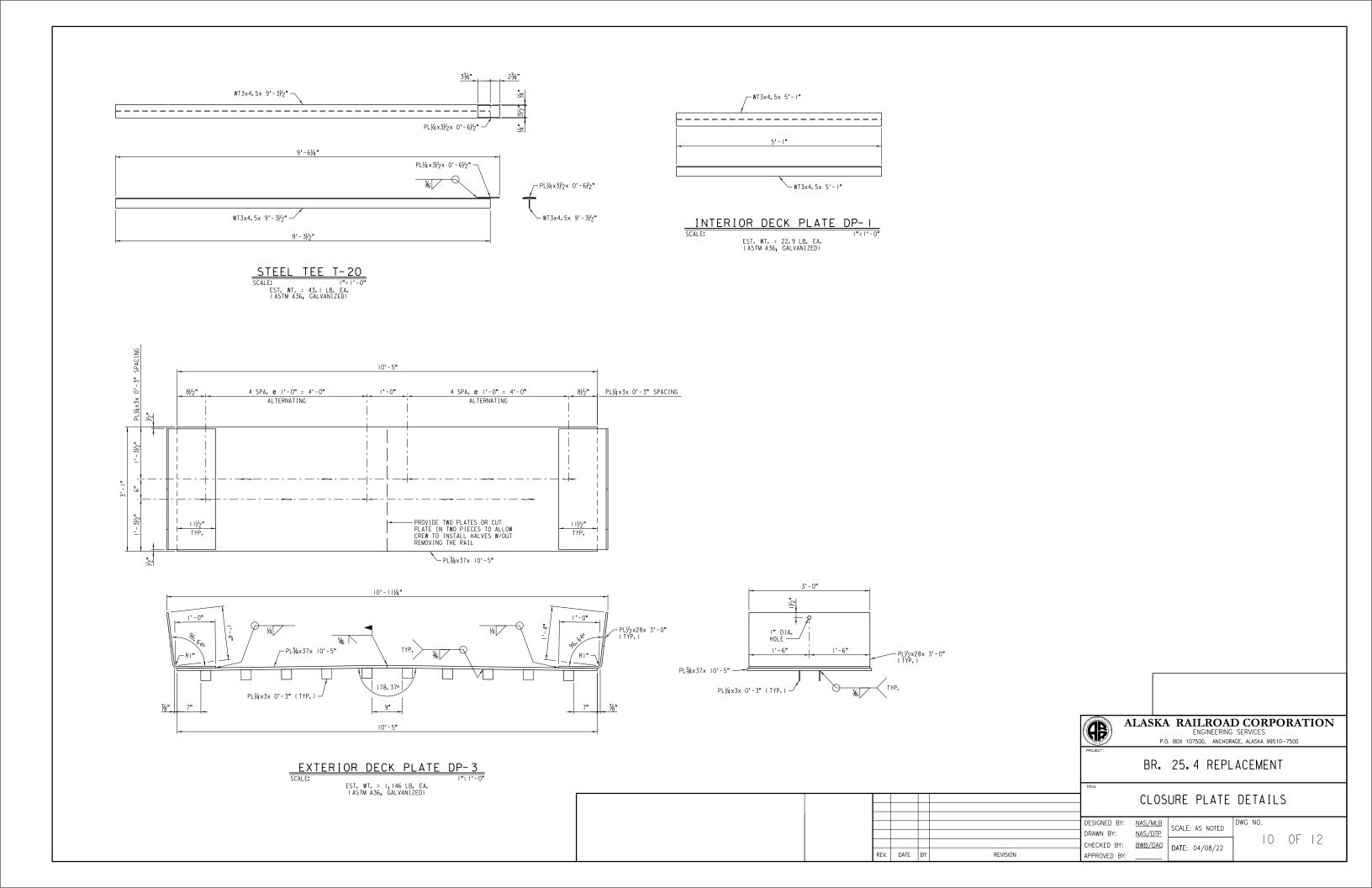
ENGINEERING SERVICES

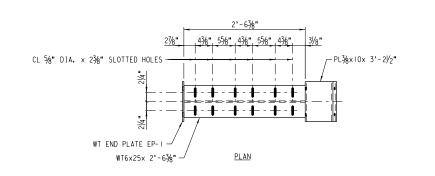
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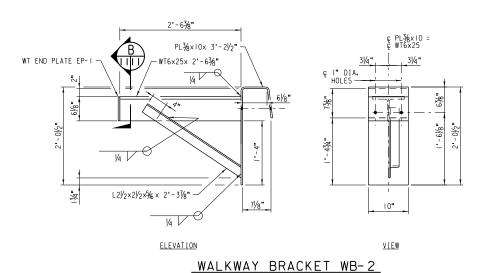
BR. 25.4 REPLACEMENT

SHEET PILE BULKHEAD DETAILS (SHEET 2 OF 2)

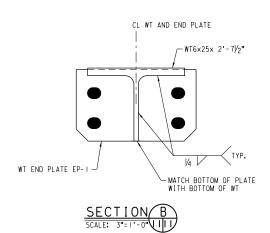
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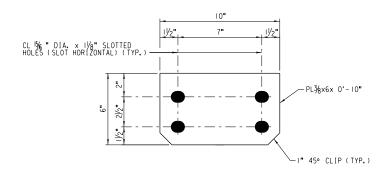






EST. WT. = 123 LB. EA.





WT END PLATE EP- I SCALE: BST. WT. = 6.4 LB. EA. 3"=1'-0"

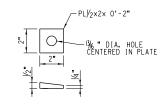
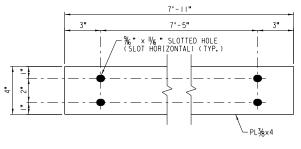


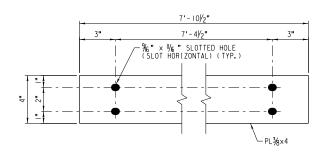
PLATE WASHER PW-1

SCALE: 3"=1'-C

EST. WT. = 0.5 LB. EA.
(ASTM A36, GALVANIZED)







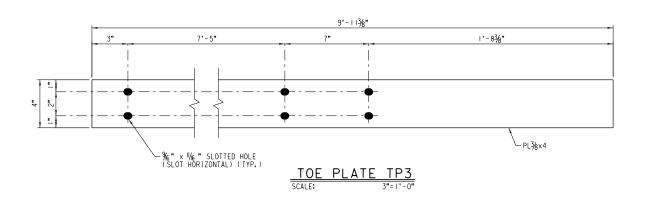
NOTES:



 STRUCTURAL STEEL PLATES, ANGLES, AND WT'S SHALL CONFORM TO ASTM A709 GRADE 36.

2. WALKWAY BRACKET WB-2 SHALL BE GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A123.

3. AFTER GALVANZING ALL LEMENTS SHALL BE FREE OF FINS, ABRASIONS, ROUGH OR SHARP EDGES AND OTHER SURFACE DEFECTS.





BR. 25.4 REPLACEMENT

MISCELLANEOUS STEEL DETAILS (SHEET | OF 2)

 DESIGNED BY:
 NAS/MLB NAS/DTP
 SCALE: AS NOTED

 DRAWN BY:
 NAS/DTP
 DATE: 04/08/22

 APPROVED BY:
 NAS/DTP
 DATE: 04/08/22

II OF 12

